SEP **1 2** 2016



CITY OF CALISTOGA

ONSITE WASTEWATER DISPERSAL FEASIBILITY STUDY FOR AUBERT WINERY 333 SILVERADO TRAIL, CALISTOGA, CA 94515 APN 011-050-031

As required by Napa County Planning, Building and Environmental Services (PBES) and the City of Calistoga, this study outlines the feasibility of providing onsite wastewater dispersal for Aubert Winery located on the above reference parcel.

PROJECT DESCRIPTION

The project proposes an increase in wine production from 23,700 gallons annually (10,000 cases) to 35,550 gallons annually (15,000 cases) along with modification to the existing onsite wastewater dispersal system serving the existing tasting room and full crush winery located on the 2.0± acre subject parcel. Refer to the associated Use Permit Drawings prepared by Bartelt Engineering for the existing conditions and proposed improvements.

The project proposes an increase in wine production only and does not propose a modification to the existing marketing plan, which includes private tour and tasting appointments for a maximum number of 25 guests per day. Furthermore, a modification to the existing winery events schedule and staffing plan, which includes five (5) full-time employees and four (4) part-time employees without any seasonal (harvest) employees, is not proposed at this time.

Table 1 summarizes the staffing plan:

| TABLE 1: STAFFING PLAN SUM | MARY |
|----------------------------|------------------------------------|
| Description | Staff Members Existing/Proposed |
| Full-Time Employee | 5 |
| Part-Time Employee | 4 |
| Harvest Season Employee | 0 |

Table 2 summarizes the marketing plan:

| TABLE 2: MARKETING PLAN | N SUMMARY | | | |
|-------------------------|-----------|---------------------|-----------|------------------|
| | Exi | sting | Prop | osed |
| Description | Frequency | Number of Guests | Frequency | Number of Guests |
| Private Tours & Tasting | Daily | 25 per day | Daily | 25 per day |
| Winery Events | Annually | 0 per event | Annually | 0 per event |



As part of our services, representatives from Bartelt Engineering have reviewed the planned operational methods for the winery with our Client, reviewed the parcel files at PBES and the City of Calistoga's Planning, Building and Public Works Department, held conversations with PBES and City of Calistoga staff, performed a reconnaissance of the site to view existing conditions and conducted several site visits to evaluate the feasibility of replacing the existing onsite wastewater treatment system.

This study and the associated Use Permit Drawings demonstrate that the proposed winery improvements and wine production increase can feasibly be developed and that all wastewater can be adequately disposed of.

WASTEWATER ANALYSIS

All plumbing fixtures in the winery production facility and tasting room will be water saving fixtures per the California Plumbing Code as adopted by the City of Calistoga Building Department.

Winery Production Process Wastewater Flow

The winery production process wastewater (PW) flow rates for harvest and non-harvest seasons can be calculated as follows:

Harvest Peak Winery PW Flow=

$$\left(\frac{35,550 \text{ gallons of wine}}{\text{year}}\right) \times \left(\frac{1.5 \text{ gallons of water}}{1 \text{ gallon of wine}}\right) \times \left(\frac{1 \text{ year}}{40 \text{ days of crush}}\right) =$$

Harvest Peak PW Flow = 1,333 gallons per day (gpd)

Non-Harvest Peak PW Flow=

$$\left(\frac{35,550 \text{ gallons of wine}}{\text{year}}\right) \times \left(\frac{4.5 \text{ gallons water}}{1 \text{ gallon of wine}}\right) \times \left(\frac{1 \text{ year}}{325 \text{ days}}\right) =$$

Non-Harvest Peak PW Flow = 492 gpd

Winery Sanitary Wastewater Flow

The sanitary wastewater (SW) generated at the winery production facility and tasting room including full-time employees, part-time employees and guests can be itemized as follows:

Employees:

| | 5 Full-Time Employees x 15.0 gpd per employee = | 75 gpd |
|---|---|--------|
| • | 4 Part-Time x 15.0 gpd per employee = | 60 gpd |
| • | 0 Harvest Season x 15.0 gpd per employee = | 0 gpd |



Guests¹:

- Tours and Tasting:
 - o (25 guests per day) x (3 gpd per guest) =

75 gpd

The proposed peak flow is the combination of SW generated from employees and tasting room guests. Since harvest employees are not being proposed, SW flows are likely to be consistent year round.

Design Wastewater Flows

The greatest practical harvest and non-harvest season peak daily process and sanitary wastewater flows are summarized in the following table:

| TABLE 3: HARVEST AND NON-HA | arvest Season Peak Daily Flow | SUMMARY |
|------------------------------------|-------------------------------|-----------------------------|
| Wastewater Source | Harvest (gpd) | Non-Harvest (gpd) |
| Winery Sanitary Wastewater (SW) | 210 | 210 |
| Winery Process Wastewater (PW) | 1,333 | 492 |

The greatest wastewater flow from each stream is used for sizing the wastewater improvements. As summarized in the above table, the PW treatment system is proposed to have a peak design flow of 1,333 gpd and SW is proposed to have a peak discharge flow rate of 210 gpd.

Portable toilets will be utilized onsite during any temporary events to prevent sanitary wastewater flows from exceeding the peak daily amount shown in the above table.

WASTEWATER TREATMENT AND DISPERSAL METHODS

The PW and SW streams are currently treated and dispersed in an existing combined onsite wastewater treatment system (OWTS). As part of proposed improvements the PW and SW streams are proposed to be handled separately. The proposed improvements are discussed further in the following sections as well as summarized in the attached wastewater treatment diagrams.

Existing Wastewater System

The existing OWTS was originally designed in 2003 by Sterk Engineering, Inc. for a peak design flow of 950 gpd. The existing OWTS is reported to disperse PW and SW septic tank effluent (STE) through a pressure distribution (PD) dispersal field. The existing dispersal field consists of 560 lineal feet of PD laterals. A 6,210 square foot replacement area is also designated on the parcel for an evapotranspiration-infiltration (ETI) bed.

¹ Volume rate accounts for 3 gpd for restroom use



Proposed Process Wastewater System Options

Bartelt Engineering proposes to abandon and remove the existing OWTS. Several options for treatment and dispersal of winery process wastewater are proposed. An option will be selected for installation following approval of the Use Permit modification.

The winery facility's PW system is proposed to consist of several steps. The floors of the expanded winery and work area would be sloped so that all PW is collected in trench drains and floor drains. The drains would be fitted with baskets to collect a majority of the larger debris. PW collected in the existing winery building would combine with PW collected in the expanded winery building and flow by gravity to the proposed pretreatment system or holding tank.

Subsurface Drip Dispersal System with Pretreatment and Replacement System

Under this preferred option, PW collected from the winery would be pretreated prior to dispersal through a subsurface drip field. Examples of a pretreatment system include (but not limited to) Cloacina, AdvanTex or Lyve Systems. The pretreatment system selected for installation is anticipated to include a pH adjustment system, primary treatment tank equipped with an aeration system and a membrane or media filtration system.

Sizing of the subsurface drip field is based on a reported soil condition of Sandy Loam (SL) with acceptable soil to 72 inches as documented in the attached site evaluations². Soil hydraulic loading rates are based on Napa County Standards and dripline manufacturer's requirements. For SL soil, Napa County recommends a soil hydraulic loading rate of 1.0 gallons per square feet per day³. The proposed dripline manufacturer GeoFlow Incorporated, recommends a hydraulic loading rate of 0.9 gallons per square feet per day⁴. The subsurface drip field area is calculated based on the design flow and lower hydraulic loading rate, as shown below:

Subsurface Drip Dispersal Field Area =
$$\frac{\text{design flow rate}}{\text{hydraulic loading rate}} = \frac{1,333 \frac{\text{gal}}{\text{day}}}{0.9 \frac{\text{gal}}{\text{day/ft}^2}} = 1,482 \text{ ft}^2$$

Site slopes in the proposed subsurface drip field area are greater than 20%. Therefore, three (3) foot spacing is recommended between drip lines per Napa County Standards. Total subsurface drip area recommended is 2,223 square feet.

Per Napa County standards, a 200% subsurface drip replacement area is provided and calculated below:

Replacement Area = $200\% \times 1,482 \text{ ft}^2 = 2,964 \text{ ft}^2$

² Site evaluations performed by Bruce Sakai General Engineering on April 2002 and October 25, 2002 and recorded by Napa County Environmental Health staff.

³ Hydraulic loading rate is based on *Table III-2 Soil Hydraulic Loading Rates* from Napa County Onsite Wastewater Treatment Systems (OWTS) Technical Standards, Final Draft

⁴ Hydraulic loading rate is based on *Table 1 Drip Loading Rates Considering Soil Structure* from The Subsurface Drip Dispersal and Reuse Design, Installation and Maintenance Guidelines prepared by GeoFlow Incorporated, October 2007.



Based on site slopes in the proposed replacement area exceeding 20%, a minimum replacement subsurface drip field area of 4,446 square feet is recommended.

Refer to the associated Use Permit Drawings for location of the proposed subsurface drip dispersal field and replacement area.

Surface Drip Irrigation with Pretreatment

Under this alternate option, PW collected from the winery would also be pretreated prior to being beneficially reused for surface drip irrigation on designated areas where ground slopes do not exceed 30%. A PW flow balance was determined by estimating the monthly PW produced (see Table I), the average irrigation flow based on estimated turf grass irrigation demands (see Table II) and sizing a storage tank to be able to store excess treated PW effluent until it can be properly dispersed via surface drip irrigation throughout designated landscaped/turf areas (see Table IV).

The combined treated PW storage tank(s) have a minimum required volume of 78,000 gallons. Actual volume recommended is 90,000 gallons, which consists of three (3) new 30,000 gallon tanks and two (2) repurposed existing 10,500 gallon tanks (see attached Table III). These tanks provide storage of treated effluent through the winter months when surface drip land application is minimal and to equalize differences between the wastewater generation rate and the irrigation application rate. In the months where the irrigation demand exceeds the amount of treated effluent that is available for irrigation, it is assumed that the entire irrigation requirement for the landscape/turf areas is not met or that another water source (existing onsite well) is used to supply additional irrigation water.

Landscaped/turf areas where treated PW is dispersed through surface drip irrigation is based on 0.25 minimum acres located on the subject parcel. The surface dispersal area will be verified once all dispersal field setbacks are determined and a final landscape plan is provided by the Landscape Architect/Designer. Furthermore, all surface dispersal field areas are proposed to be labeled with signage indicating the use of treated wastewater for irrigation in accordance with PBES and City of Calistoga standards.

City of Calistoga Sewer Connection Replacement Option

Connection to the City of Calistoga sewer main is another recommended replacement option. Following pretreatment, PW would connect into the existing City of Calistoga sewer main that is just south of the eastern property line and within a 15 foot wide easement. An application for the proposed connection would be filed separately with the City of Calistoga. All fees and necessary monitoring requirements would be fulfilled under this replacement option.

Hold & Haul System and Replacement Area

Under this proposed alternative option, PW from the winery would be collected and stored onsite prior to being hauled offsite to an approved wastewater treatment plant (East Bay MUD or equivalent) by a Napa County PBES approved septage hauler. As summarized in Table 3, the PW Hold & Haul system is proposed to be designed for a peak daily flow of 1,333 gpd. As part of the Hold & Haul system, an approved onsite dispersal





system must be designated for winery PW as a replacement area. Options for the replacement area include the subsurface drip dispersal system and surface drip irrigation These replacement options are all discussed in more detail in the above sections.

Proposed Sanitary Wastewater System

Bartelt Engineering proposes to abandon the existing combined OWTS and discharge SW to the existing City of Calistoga municipal wastewater system. SW from the winery and tasting room building would connect to the existing City of Calistoga sewer main that is just south of the eastern property line and within a 15 foot wide easement. An application for the proposed connection would be filed separately with the City of Calistoga.

TANK SIZING

Existing Tanks

Existing collection and processing tanks include one (1) 1,500 gallon process wastewater tank, one (1) 1,200 gallon sanitary wastewater septic tank and one (1) 1,200 gallon sump tank. The existing tanks could either be incorporated into the proposed PW treatment system if feasible during the design phase, abandoned in place per Napa County Standards or removed and disposed properly offsite.

Additional tanks onsite include one (1) designated 10,500 gallon irrigation storage tank and one (1) designated 10,500 gallon fire protection tank. Both tanks are currently fed from the existing well. The existing tanks are proposed to be repurposed for landscape/turf irrigation storage.

Process Wastewater Proposed Tanks

Sizing for the proposed PW treatment tanks are based on typical tank sizing requirements for pretreatment systems. The configuration and sizing requirements may change upon selection of a manufacturer for installation. One or more of the treatment tanks could be combined into one larger compartmentalized tank if desired during the design phase.

Equalization Tank

The pretreatment system is proposed to be preceded by an equalization (EQ) tank for buffering of peak flows. The proposed EQ tank is sized to provide a minimum of three (3) days of hydraulic retention time and available volume to store decanted sludge during peak flow conditions. A fine bubble diffused air system should also be provided to keep PW adequately mixed prior to entering the primary treatment tank.

Screen or Settling Tank

A screen or settling tank may be added (if recommended by the pretreatment system manufacturer) for removal and/or settling of solids prior to entering the primary treatment tank.



Primary treatment tank

The pretreatment system manufacturer selected for installation will size the primary treatment tank, aeration system, membrane filtration system and effluent pump. The pretreatment system manufacturer may also use chemical additions for pH adjustment and nutrient additions to promote biological growth and improve treatment removal rates.

Hold & Haul Tank

Per Napa County PBES requirements, the Hold & Haul tank system must be able to store a minimum of seven (7) days of peak PW flows. The tank can be either an above ground or below grade holding tank. If an above ground tank is selected for installation, secondary containment complying with PBES requirements must be provided.

Hold & Haul Storage Tank

= 7 days x 1,333 gpd

= 9,331 gallons, 10,000 gallons recommended

Operation and Maintenance

Per Napa County requirements, all alternative sewage treatment systems (ASTS) including winery wastewater treatment systems with pretreatment are required to have a Service Provider. The Service Provider can be a Registered Civil Engineer, Registered Environmental Health Specialist or Licensed Contractor. The PW pretreatment system manufacturer can also provide operation and maintenance services for their own system. The Service Provider would be assigned prior to operation and final approval of the installed PW system.

SURROUNDING FEATURES

Based on research and knowledge of the surrounding area, there does not appear to be any caves located within 400 feet from the proposed PW dispersal areas. The City of Calistoga 1.5 million gallon municipal water storage tank is located on Mount Washington which is located at a higher elevation and 600± feet from the proposed PW dispersal area.

CONCLUSIONS

Process wastewater generated as a result of the proposed improvements, which includes expansion of the existing winery and tasting room building as well as an increase in wine production, can feasibly be treated and dispersed onsite in accordance with Napa County PBES and City of Calistoga standards. Sanitary wastewater, which is not projected to increase as a result of the proposed improvements, can feasibly be discharged to the existing City of Calistoga sewer main to be treated at the municipal wastewater treatment plant.

Full design calculations and construction plans will be completed after approval of the Use Permit Modification under consideration.





ATTACHMENTS

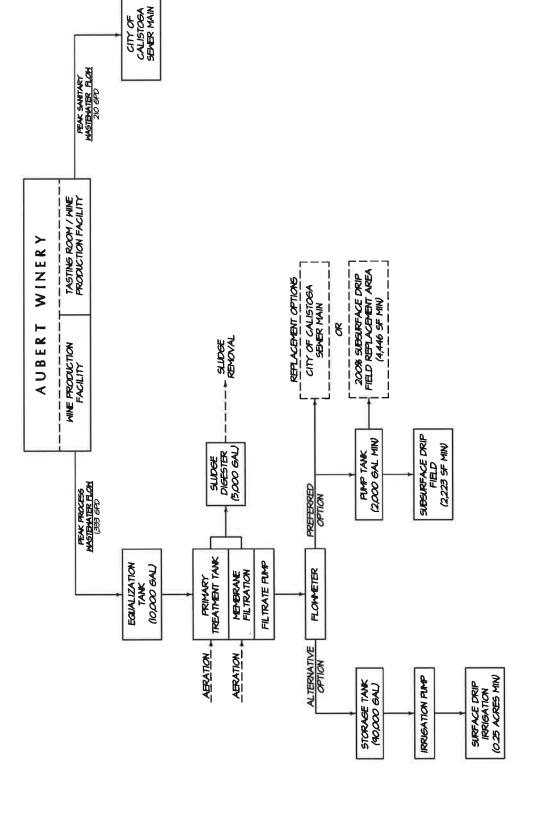
Proposed Wastewater Treatment Diagrams
Table I – Process Wastewater Flow
Table II – Monthly Rainfall Rates
Table III – Process Wastewater Irrigation
Table IV – Process Wastewater Irrigation Storage Tank Balance
Site Evaluations



REFERENCES

- California Onsite Wastewater Association (COWA). "Pumping and Pressure Distribution Systems." May 1998.
- Geoflow, Inc. "Wastewater Design, Installation and Maintenance Guidelines." v1, 2007.
- Napa County Department of Environmental Management. "Design, Construction and Installation of Alternative Sewage Treatment Systems." April 12, 2010.
- Telsco Industries. "Turf Irrigation Manual." By James A. Watkins. 1987.
- U.S. Department of Health, Education and Welfare, Public Health Service Publication.

 Manual of Septic-Tank Practice. 1967.
- U.S. Environmental Protection Agency. "Onsite Wastewater Treatment Systems Manual." April 2002.
- Napa County Planning, Building and Environmental Services, "Napa County Onsite Wastewater Treatment Systems (OWTS) Technical Standards." Final Draft.



PROPOSED WASTEWATER TREATMENT DIAGRAM

NOT TO SCALE

E N.G. I N. E E R. I. N. G.
CYNL ENCNERING - LAND PLANNING
THIS Jefferom Street, 200 B, Napa, CA 94559
www.bartelkengineering.com
. Telephoner 707/258-1301 ·

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PEAK SANITARY MASTEMATER FLOM 210 GPD CITY OF CALISTOGA SEMER MAIN TASTING ROOM / WINE PRODUCTION FACILITY WINERY AUBERT WINE PRODUCTION FACILITY OFFSITE REMOVAL ALTERNATIVE OPTION

SURFACE DRIP IRRIGATION

8

CITY OF CALISTOGA SEWER MAIN

8

SUBSURFACE DRIP FIELD REPLACEMENT OPTIONS

SEPTAGE HAULER TRUCK

HOLDING TANK (10,000 GAL)

PEAK PROCESS
WASTEWATER FLOW
1333 GFD

PROPOSED WASTEWATER TREATMENT DIAGRAM NOT TO SCALE

Aubert Winey
333 Silverado Trail
Calistoga, CA 94515
APN 071-050-031
Job No. 14-41
September 2016
Sheet 2 of 2

BAREERING

ENGINEERING

CAMENGNERING

CAMENGNERING

CAMENANANG

Telephone 707-259-1301

CAPPAGNET 2016, ALL REHTS RESERVED.

Aubert Winery Process Wastewater Flow Table I



| Total annual wine production (cases): | 15,000 |
|--|---------|
| Gallons per case of wine: | 2.37 |
| Total annual wine production (gallons): | 35,550 |
| Annual water usage per gallon of wine (gallons): | 6 |
| Annual process wastewater flow (gallons): | 213,300 |
| Average annual process wastewater flow (gpd): | 584 |
| | |
| Harvest water usage per gallon of wine (gallons): | 1.5 |
| Duration of Harvest (days): | 40 |
| Harvest process wastewater flow (gallons per day): | 1,333 |
| Non-harvest water usage per gallon of wine (gallons): | 4.5 |
| Duration of Non-Harvest (days): | 325 |
| Non-harvest process wastewater flow (gallons per day): | 492 |

MONTHLY PROCESS WASTEWATER FLOW (gallons/month):

| ESTIMATED PROCESS WAST | EWATER FLOW | |
|----------------------------------|-------------|-----------------|
| Month | Percent | Wastewater Flow |
| September (Harvest Month) | 19.6% | 41,807 |
| October (End of Harvest Season) | 14.3% | 30,502 |
| November | 4.3% | 9,172 |
| December | 6.6% | 14,078 |
| January | 2.1% | 4,479 |
| February | 1.8% | 3,839 |
| March | 2.6% | 5,546 |
| April | 3.6% | 7,679 |
| May | 7.0% | 14,931 |
| June | 10.6% | 22,610 |
| July | 11.6% | 24,743 |
| August (Start of Harvest Season) | 15.9% | 33,915 |
| TOTALS | 100.0% | 213,300 |

Notes:

- > Wastewater monthly proportioning is based on winery water usage data from 2012-2014 provided by Aubert Winery
- > The annual water usage per gallon of wine is assumed to be 6 gallons

Aubert Winery Monthly Rainfall Rates Table II



| Monthly Rainfa | ll Rates | |
|----------------|-----------------------|-----------------------|
| | Site | 10-year |
| | Rainfall ¹ | Rainfall ² |
| Month | (in) | (in) |
| September | 0.5 | 0.70 |
| October | 2.0 | 2.80 |
| November | 4.6 | 6.44 |
| December | 6.3 | 8.82 |
| January | 8.2 | 11.48 |
| February | 6.7 | 9.38 |
| March | 5.6 | 7.84 |
| April | 2.2 | 3.08 |
| May | 1.0 | 1.40 |
| June | 0.3 | 0.42 |
| July | 0.1 | 0.14 |
| August | 0.1 | 0.14 |
| TOTALS | 37.6 | 52.64 |

¹⁾ Site rainfall from Calistoga, CA (NCDC Cooperative Stations 1990-1995); see www.worldclimate.com

^{2) 10} year rainfall = Site rainfall \times 1.4

Process Wastewater Irrigation Aubert Winery Table III



Irrigation Method:

Surface Drip

Ladscape/turfgrass Area (acres):

| | | | Estimated Orchard | Estimated Orchard Process Wastewater Irrigation | tion | | |
|-----------|------|---------------------|-------------------|---|--|---------------------|----------------------|
| | | Evapotranspiration, | Turfgrass | Turfgrass | Precipitation | Crop Irrigation | Total Irrigation (6) |
| | | ETo ¹ | Coefficient, Kc2 | Evapotranspiration, ETc ³ | 10-year, i _{10-yR} ⁴ | Demand ⁵ | Available |
| Month | Days | (in) | | (in) | (in) | (in) | (gallons) |
| September | 30 | 5.1 | 08.0 | 4.08 | 0.70 | 3.38 | 22,945 |
| October | 31 | 3.4 | 0.80 | 2.73 | 2.80 | 0.00 | 0 |
| November | 30 | 1.8 | 08.0 | 1.44 | 6.44 | 0.00 | 0 |
| December | 31 | 6.0 | 0.80 | 0.74 | 8.82 | 0.00 | 0 |
| January | 31 | 1.2 | 08.0 | 0.99 | 11.48 | 0.00 | 0 |
| February | 28 | 1.7 | 08'0 | 1.34 | 9.38 | 0.00 | 0 |
| March | 31 | 3.4 | 0.80 | 2.73 | 7.84 | 0.00 | 0 |
| April | 30 | 4.8 | 0.80 | 3.84 | 3.08 | 0.76 | 5,159 |
| May | 31 | 6.2 | 08'0 | 4.96 | 1.40 | 3.56 | 24,167 |
| June | 30 | 6.9 | 0.80 | 5.52 | 0.42 | 5.10 | 34,622 |
| July | 31 | 7.4 | 0.80 | 5.95 | 0.14 | 5.81 | 39,455 |
| August | 31 | 6.5 | 0.80 | 5.21 | 0.14 | 5.07 | 34,405 |
| TOTALS | 365 | 7 67 | 09 6 | 39.54 | 52 64 | 24 | 160 754 |

1) Reference evapotranspiration (ETo) data for Napa County (Zone 8) is referenced from the Department of Water Resources California Irrigation Management

2) K_c coefficients for warm-season turigrasses in California from University of California, Division of Agriculture and Natural Resources Information Systems (website: www.cimis.water.ca.gov)

3) $ET_c = ET_o \times K_c$

4) 10-year precipitation = Average precipitation \times 1.4. See Rainfall Rates, Table II

5) Irrigation Demand = ET_c - 10-year precipitation

6) Total irrigation available (gallons/month) = $(No. of acres) \times irrigation demand (inches/month) / 12 (inches/foot) \times 325,853 (gallons/acre-feet)$

References:

> University of California, Division of Agriculture and Natural Resources, Center for Landscape & Urban Horticulture

> California Irrigation Management Information System (CIMIS)



Process Wastewater Irrigation Storage Tank Balance Aubert Winery Table IV



| | ESTIMA | | | | | |
|--------|-----------|------------|-----------|-------------------|------------|-----------|
| | Beginning | Wastewater | Total | Minimum | Additional | Tank |
| Month | Balance | Flow' | Volume | Irrigation Needed | Irrigation | Volume |
| | (gallons) | (gallons) | (gallons) | (gallons) | (gallons) | (gallons) |
| tember | 0 | 41,807 | 41,807 | 22,945 | 11,012 | 7,849 |
| ctober | 7,849 | 30,502 | 38,351 | 0 | 0 | 38,351 |
| vember | 38,351 | 9,172 | 47,523 | 0 | 0 | 47,523 |
| cember | 47,523 | 14,078 | 61,601 | 0 | 0 | 61,601 |
| ınuary | 61,601 | 4,479 | 080'99 | 0 | 0 | 080′99 |
| bruary | 080′99 | 3,839 | 69,920 | 0 | 0 | 69,920 |
| Aarch | 69,920 | 5,546 | 75,465 | 0 | 0 | 75,465 |
| April | 75,465 | 629'2 | 83,144 | 5,159 | 0 | 77,985 |
| May | 77,985 | 14,931 | 92,916 | 24,167 | 8,500 | 60,248 |
| June | 60,248 | 22,610 | 82,858 | 34,622 | 11,012 | 37,224 |
| July | 37,224 | 24,743 | 61,967 | 39,455 | 11,012 | 11,500 |
| August | 11,500 | 33,915 | 45,415 | 34,405 | 11,012 | 0 |
| | TOTALS | 213,300 | | 160,754 | 52,548 | |
| | AVERAGE | 17,775 | | 13,396 | | 46,146 |

Minimum Tank Volume (gallons): Reccomended Tank Volume (gallons): Reccomended Tank Volume (acre-feet):

78,000 90,000 0.28

Refer to Table I for Wastewater Flows
 Refer to Table III for Total Irrigation Available

Tank Balance Surface Drip Option



RAPA COUNTY DEPARTMENT OF ENVIRONMENTAL MANAGEMENT REQUEST FOR SITE EVALUATION INSPECTION

SER

| THE OWN I | rec. # 92-13910 |
|--------------------------------------|---|
| ENVIRONMENTAL HEALTH DEPT. USE ONLY | PARCEL NUMBER: 011-050-031 |
| fee: 348.00 | PARCEL NUMBER: 011-030-031 |
| DATE: 2-4-02 | OWNER: JOE BRIGGS RESONT DEV. CO. |
| RECEIPT: 2/13-5 | OWNER: JOE BRIGGS RESORT DEV. CO. |
| ву: | TEST CONDUCTED BY: Bruce SAKAI GENERAL ENG. |
| | 942-0578 |
| TYPE OF TEST: FIELD ANALYSIS | PERCOLATION TEST |
| rei 3/1/02 | To be run onfromam/pm topm |
| DUBDOCK OF TECT. HOUSE. | WINERY: X OTHER: 24,000 x 15/45 = 800 gpd. |
| PROJECTED WASTEWATER FLOWS: 2 4,000 | gpd Jagob Spy Alf gpd |
| **** | ************************************** |
| Pre-soak checked? yes no | Length of pre-soak: |
| Checked by: | Date: |
| Rate at time of inspection: | Stabilized perc rate: |
| Gravel and Pipe Used? yes no | If so, take the perc rate x .6 =in/hr |
| ********* | ************************************** |
| STANDARD SYSTEM | "- RESERVE AREA |
| Acceptable soil to: 72 / As | 11- RESERVE ANDA signed perc range: 1-3 / 3-6 / 6-12 |
| Depth of trenches: 36 / Ro | ck under pipe: /B / Cover over rock: /Z |
| Lineal feet of leachline required: | problems: (800 Reg for Reserve Area w/ 30" frenchs and 12" rock under the pipe.) |
| Slope: 0- 15% Surface drainage | problems: (800 Reg For Reserve Ares w/ 30" brends |
| Additional information: | and 12 rock under the pipe. |
| | |
| SPECIAL DESIGN SYSTEM DUE TO THE FOL | LOWING - Size constraints: |
| Perc rate too slow:/Per | c rate too fast:/Steep slope: |
| | /High seasonal groundwater: |
| Acceptable soil for special design: | /Other problems: |
| E.H. Specialist Muld | Bate 3/1/02 |

FIELD ANALYSIS

SAND CONTENT GRAVEL, COBBLE, STONE CONTENT

TEXTURE (In the proposed treuch zone)

CLAY CONTENT

| Low (<12) |
|--|
| High (27-40) High (>40) *************************** STRUCTURE SOIL DENSITY WHEN PICKED (Circle whether wet or dry) Core Hole pick sluffs or caves soil in pick bites and soil sluffs pick bites/ little or no soil sluffs STRUCTURE STRUCTURE MODIFIER CHARACTERISTICS MODIFIER CHARACTERISTICS MODIFIER CHARACTERISTICS |
| High (>40) * * * * * * * * * * * * * * * * * * * |
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| STRUCTURE Core Hole Granular Blocky STRUCTURE 1 2 3 4 5 6 1) Soil Survey Name: |
| Core Hole Granular Blocky 1 2 3 4 5 6 1) Soil Survey Name: |
| Core Hole Granular Blocky 1 2 3 4 5 6 1) Soil Survey Name: |
| Granular Blocky 1) Soil Survey Name: |
| Blocky |
| The desired the second to the |
| Prism 2) Horizon Boundaries: Diffuse Gradual Abrupt |
| |
| Massive 3) Topography: Concave / Aspect: |
| 4) Vegetation: Type (Condition: |
| oundition. |
| ************* |
| CORE HOLE RECORD |
| HOLE #1 EST. HOLE #2 EST. HOLE #3 EST. PERC |
| 0 to 72" PERC to TDO Blef 13 |
| 20.00 |
| To be soil to Same to More Clay |
| |
| to Possibly ZI"/nc |
| Roots: Roots: |
| Color: bright / dull Color: bright / dull Color: bright / dull |
| Water Table: 72 Water Table: 72 Water Table: |
| Dug:easy / hard / dusty /smear Dug:easy / hard / dusty / smear Dug:easy /hard /dusty /smear |
| Acceptable Soil To: 72 Acceptable Soil To: 72 Acceptable Soil To: ? |
| QEYENE POLEA CORE HOLE RECORD |
| HOLE #4 EST. HOLE #5 EST. HOLE, #6 EST. |
| PERC PERC PERC |
| 0 to 66 3-6 to to |
| 1-16 SCL / SCL |
| 6/ to Wester tible to |
| |
| to |
| |
| Roots: Roots: Roots: |
| Roots: Color: bright / dull Color: bright / dull Color: bright / dull |
| Roots: Color: bright / dull Water Table: Water Table: Roots: Color: bright / dull Water Table: Water Table: Water Table: |
| Roots: Color: bright / dull Color: bright / dull Color: bright / dull |

011-050-031

NAPA COUNTY DEPARTMENT OF ENVIRONMENTAL MANAGEMENT REQUEST FOR SITE EVALUATION INSPECTION

92-13910

SP

| addon's a | Siteral |
|--|--|
| ENVIRONMENTAL HEALTH DEPT. USE ONLY | |
| FEE: CONTINUATION | PARCEL NUMBER: 011-050-031 |
| DATE: | JOB ADDRESS: MT. WASHINGTON / S. TRAIL |
| RECEIPT: | OWNER: BEIGS |
| ву: | TEST CONDUCTED BY: SAKAI |
| | |
| TYPE OF TEST: FIELD ANALYSIS × | PERCOLATION TEST |
| To be run on 10/25/02at am | /pm To be run onfromam/pm topm |
| PURPOSE OF TEST: HOUSE: | WINERY: X OTHER: |
| PROJECTED WASTEWATER FLOWS: 24,000 | OGPY 800 spd |
| ************************************** | ************************************** |
| Pre-soak checked? yes no | Length of pre-soak: |
| Checked by: | Date: |
| | Stabilized perc rate: |
| Gravel and Pipe Used? yes no | If so, take the perc rate x .6 =in/hr |
| ********** | 表表表表表表表表表表表表表表表表表表表表表表表表表表表表表表表表表表表表表 |
| STANDARD SYSTEM | TPE OF SYSTEM APPROVED |
| | signed perc range: 143 / 3-6 / 6-12 |
| | ck under pipe:/ Cover over rock: |
| Lineal feet of leachline required: | / Plot plan received: 10/29/02 |
| Slope:/ Surface drainage | problems: |
| | (10/25/02) \$ HOLE # 2 (3/1/02) OX FOR STAND. SYS. |
| HOLE# 4 (3/1/02) OX FOR RES. NO | 4 IF MEETS WELL SET BACK. HOLES # 3 \$5 OXFOR ENG. RES. APEA. |
| SPECIAL DESIGN SYSTEM DUE TO THE FOL | The state of the s |
| Perc rate too slow:/Per | c rate too fast:/Steep slope: |
| | /High seasonal groundwater: |
| | JARIES Other problems: NOW FOR STANDARD |
| \$15TOM - HOURS \$23#50 | TOP ENG. SUS. W/O SAND, 314 NOT ACCEPTABLE |
| E.H. Specialist | 1/01-5 Date 10/25/02 |

TEXTURE (In the proposed treuch zone)

| CLAY CONTENT Core Hole | Core Hole 1 2 3 4 5 6 | RAVEL, COBBLE, STONE CONTENT ore Hole ery High (>60) |
|--|---|---|
| Mod (12-27) | Mod (20-50) → → → H Low (<20) | 1gh(35-60) od (15-35) ow (<15) **Y-X-* **T |
| * * * * * * * * * * * * * * * * * * * | * * * * * * * * * * * * * * * * | * * * * * * * * * * * * * * * * * |
| SOIL DENSITY WHEN PICKED (Core Hole | | CONSISTENCE (Circle w or d) Core Hole 1 2 3 4 5 6 |
| pick sluffs or caves soil in pick bites and soil sluffs pick bites/ little or no soil s | 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | Easy Moderate |
| STRUCTURE MODIFIER CHARACTERISTICS STRUCTURE MODIFIER CHARACTERISTICS | | |
| Core Hole 1 2 3 4 5 6 Granular | | DAM. GUBARLAKES GLAY |
| Prism Platy | 2) Horizon Boundaries: Diffuse | Gradual Abrupt X_ |
| Massive X Cemented X X | 3) Topography: Concave | Convex / Aspect: |
| 4) Vegetation: Type 6245 Condition: DM * * * * * * * * * * * * * * * * * * * | | |
| HOLE #1 EST. | HOLE #2 EST. | HOLE #3 EST. |
| 0 to 12" VERTHENT 1+3 | O to 36" VORY TIGHT 143 | PERC |
| 12" to 48" " 16HT SANDY 1-3 | 30" to 46" CEMENTED <1" | O to 24" VORY TIGHT 163 SANDY LOAM 24" to S4" CEMENTED |
| 48" to 66" VERY TIGHT 163 | 48" to BILLEMENTED 41" | to |
| Roots: 42" Color: bright? / dull | Roots: FEW / FINE Color: bright) / dull | Roots: 24" Color: bright (1) dull |
| Water Table: NO Dug: casy / hard / dusty /smear | Water Table: NO Dug:easy / hard / dusty / smear | Water Table: NO Dug:easy /hard /dusty /smear |
| Acceptable Soil To: 60" | Acceptable Soil To: 36" | Acceptable Soil To: 24" |
| HOLE #4 EST. PERC | HOLE #5 EST. | HOLE #6 EST. |
| 0 to 20" stay flour 163 | C to 40" VERY MORE 143 | to |
| 20" to 42" DRY MASSIVE (1" CLAY (SHIZVIK) | 40" to 40" CEMENTED (1" OPANGE 3 GOMY S. AND | to to to |
| \$ GDAJEST SAND 712" | to | to |
| Roots: Fine Color: bright dull | Roots: 40' Color: bclght / dull | Roots: Color: bright / dull |
| Water Table: NO Dug:easy / hard / dusty /smear Acceptable Soil To: 20" | Dug:east / hard / dusty / smear Acceptable Soil To: 40 | Water Table: Dug:easy /hard /dusty /smear Acceptable Soil To: |
| The second secon | | Acceptable 3011 10: |

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